



PAT McCRORY  
Governor

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Secretary

January 5, 2016

**Addendum No. 3**

Contract No.: C203702  
TIP No.: I-3802B / I-3610 / B-5365  
Counties: Cabarrus & Rowan  
Project Description: I-85 from north of Lane Street to north of the US 29 / US 601 Connector;  
I-85 / NC 152 and NC 152 / US 29 / US 601 Connector Interchanges;  
and Bridge Nos. 21 and 34

RE: Addendum No. 3 to Final RFP

**February 25, 2016 Letting**

To Whom It May Concern:

Reference is made to the Final Request for Proposals dated November 6, 2015 recently furnished to you on the above project. We have since incorporated changes, and have attached a copy of Addendum No. 3 for your information. Please note that all revisions have been highlighted in gray and are as follows:

The first and third pages of the *Table of Contents* have been revised. Please void the first and third pages in your proposal and staple the revised first and third pages thereto.

Page No. 5 of the *Submittal of Quantities, Fuel Base Index Price and Opt-Out Option Project Special Provision* has been revised. Please void Page No. 5 in your proposal and staple the revised Page No. 5 thereto.

Page No. 48 of the *Price Adjustments for Asphalt Binder Project Special Provision* has been revised. Please void Page No. 48 in your proposal and staple the revised Page No. 48 thereto.

Page Nos. 329, 334, 335 and 336 of the *Materials Standard Special Provision* have been revised. Please void Page Nos. 329, 334, 335 and 336 in your proposal and staple the revised Page Nos. 329, 334, 335 and 336 thereto.

Page No. 375 has been revised to add the *Rock and Broken Pavement Fills Standard Special Provision*. Please void Page No. 375 in your proposal and staple the revised Page No. 375 thereto.

Page Nos. 375A, 375B, 375C, 375D, 375E, and 375F have been added to add the *Geosynthetics Standard Special Provision*. Please add Page Nos. 375A, 375B, 375C, 375D, 375E, and 375F to your proposal.



If you have any questions or need additional information, I can be reached by telephone at (919) 707-6900.

Sincerely,

A handwritten signature in black ink, appearing to read 'R.A. Garris', with a long horizontal flourish extending to the right.

R.A. Garris, PE  
Contract Officer

RAG/dth

cc: Mr. Rodger Rochelle, PE  
Mr. Pat Ivey, PE  
Ms. Teresa Bruton, PE  
Mr. Ron McCollum, PE  
Mr. David Hering, PE  
File

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**(B) Base Index Price**

The Design-Build Team's Estimate of Quantities will be used on the various partial payment estimates to determine fuel price adjustments. The Design-Build Team shall submit a payment request for quantities of work completed based on the work completed for that estimate period. The quantities requested for partial payment shall be reflective of the work actually accomplished for the specified period. The Design-Build Team shall certify that the quantities are reasonable for the specified period. The base index price for DIESEL #2 FUEL is **\$ 1.1964** per gallon.

**(C) Opt Out of Fuel Price Adjustment**

If the Design-Build Team elects not to pursue reimbursement for Fuel Price Adjustments, a quantity of zero shall be entered for all quantities in the *Fuel Usage Factor Chart and Estimate of Quantities* sheet and the declination box shall be checked. Failure to complete this form will mean that the Design-Build Team is declining the Fuel Price Adjustments for this project.

**(D) Change Option**

The proposer will not be permitted to change the option after the Price Proposal and the copy of the *Fuel Usage Factor Chart and Estimate of Quantities* sheet are submitted.

**(E) Failure to Submit**

Failure to submit the completed *Fuel Usage Factor Chart and Estimate of Quantities* sheet separately and in the Price Proposal will result in the Technical and Price Proposal being considered irregular by the Department and the Technical and Price Proposal may be rejected.

**INDIVIDUAL MEETINGS WITH PROPOSERS**

(9-1-11)

DB1 G048

The Department will provide at least two Question and Answer Sessions to meet with each proposer individually to specifically address questions regarding the draft Requests for Proposals.

The Department will attempt to arrange for a meeting between each individual proposer and the affected utility owners.

The Department will afford each proposer one additional meeting with the Department (maximum two-hour time limit) to discuss project specifics and address the proposer's concerns and questions. The meeting may occur at any time after the first Question and Answer Session with the proposers and before two weeks prior to the Technical and Price Proposals submittal date. The proposer shall request this meeting in writing to the State Contract Officer, providing the Department a minimum of one week advance notice of the requested date. The proposer shall also state in the request those disciplines within the Department that are requested to be in

**Submittals for Review During Construction**

The Design-Build Team shall submit the unconfined compressive strength test results for review and acceptance.

**PRICE ADJUSTMENTS FOR ASPHALT BINDER**

(9-1-11)

DB6 R25

Price adjustments for asphalt binder for plant mix will be made in accordance with Section 620 of the 2012 *Standard Specifications for Roads and Structures*.

When it is determined that the monthly selling price of asphalt binder on the first business day of the calendar month during which the last day of the partial payment period occurs varies either upward or downward from the Base Price Index, the partial payment for that period will be adjusted. The partial payment will be adjusted by adding the difference (+ or -) of the base price index subtracted from the monthly selling price multiplied by the total theoretical quantity of asphalt binder authorized for use in the plant mix placed during the partial payment period involved.

The base price index for asphalt binder for plant mix is \$401.43 per ton.

This base price index represents an average of F.O.B. selling prices of asphalt binder at supplier's terminals on January 1, 2016.

**PRICE ADJUSTMENTS - ASPHALT CONCRETE PLANT MIX**

(9-1-11) (Rev. 3-13-13)

DB6 R26

Revise the 2012 *Standard Specifications for Roads and Structures* as follows:

**Page 6-18, Article 609-11 and Page 6-35, Article 610-14**

Add the following paragraph before the first paragraph:

The "Asphalt Price" used to calculate any price adjustments set forth in this section shall be \$40 per theoretical ton. This price shall apply for all mix types.

**FIELD OFFICE**

(6-1-07) (Rev. 8-3-15)

DB 08-01

**Description**

This work consists of furnishing, erecting, equipping, and maintaining a field office for the exclusive use of Department Engineers and Inspectors at a location on the project approved by the Engineer. Provide a field office that complies with the current A.D.A. Design and Accessibility Standards, the National Electric Code, local, state, and federal regulations, and the following:

**STREET SIGNS AND MARKERS AND ROUTE MARKERS**

(07-01-95)

DB9 R01

Move any existing street signs, markers, and route markers out of the construction limits of the project and install the street signs and markers and route markers so that they will be visible to the traveling public if there is sufficient right of way for these signs and markers outside of the construction limits.

Near the completion of the project and when so directed by the Engineer, move the signs and markers and install them in their proper location in regard to the finished pavement of the project.

Stockpile any signs or markers that cannot be relocated due to lack of right of way, or any signs and markers that will no longer be applicable after the construction of the project, at locations directed by the Engineer for removal by others.

The Design-Build Team shall be responsible to the owners for any damage to any street signs and markers or route markers during the above described operations.

**MATERIALS**

(2-21-12) (Rev. 1-4-16)

1000, 1002, 1005, 1018, 1024, ~~1050, 1074~~, 1078, 1080, 1081, 1086, 1084, 1087, 1092

DB10 R01

Revise the 2012 *Standard Specifications for Roads and Structures* as follows:

**Page 10-1, Article 1000-1, DESCRIPTION, lines 9-10**, replace the last sentence of the first paragraph with the following:

Type IL, IP, IS or IT blended cement may be used instead of Portland cement.

**Page 10-1, Article 1000-1, DESCRIPTION, line 14**, add the following:

If any change is made to the mix design, submit a new mix design (with the exception of an approved pozzolan source change).

If any major change is made to the mix design, also submit new test results showing the mix design conforms to the criteria. Define a major change to the mix design as:

- (1) A source change in coarse aggregate, fine aggregate or cement.
- (2) A pozzolan class or type change (e.g. Class F fly ash to Class C fly ash).
- (3) A quantitative change in coarse aggregate (applies to an increase or decrease greater than 5%), fine aggregate (applies to an increase or decrease greater than 5%), water (applies to an increase only), cement (applies to a decrease only), or pozzolan (applies to an increase or decrease greater than 5%).

**Page 10-40, Tables 1018-1 and 1018-2, PIEDMONT, WESTERN AND COASTAL AREA CRITERIA FOR ACCEPTANCE OF BORROW MATERIAL**, under second column in both tables, replace second row with the following:

Acceptable, but not to be used in the top three feet of embankment or backfill

**Page 10-46, Article 1024-1, PORTLAND CEMENT, line 33**, add the following as the ninth paragraph:

Use Type IL blended cement that meets AASHTO M 240, except that the limestone content is limited to between 5 and 12% by weight and the constituents shall be interground. Class F fly ash can replace a portion of Type IL blended cement and shall be replaced as outlined in Subarticle 1000-4(I) for Portland cement. For mixes that contain cement with alkali content between 0.6% and 1.0% and for mixes that contain a reactive aggregate documented by the Department, use a pozzolan in the amount shown in Table 1024-1.

**Page 10-46, Table 1024-1, POZZOLANS FOR USE IN PORTLAND CEMENT CONCRETE**, replace with the following:

<b>TABLE 1024-1 POZZOLANS FOR USE IN PORTLAND CEMENT CONCRETE</b>	
<b>Pozzolan</b>	<b>Rate</b>
Class F Fly Ash	20% - 30% by weight of required cement content with 1.0 pound Class F fly ash per pound of cement replaced
Ground Granulated Blast Furnace Slag	35% - 50% by weight of required cement content with 1.0 pound slag per pound of cement replaced
Microsilica	4% - 8% by weight of required cement content with 1.0 pound microsilica per pound of cement replaced

**Page 10-47, Subarticle 1024-3(B), Approved Sources, lines 16-18**, replace the second sentence of the second paragraph with the following:

Tests shall be performed by AASHTO’s designated National Transportation Product Evaluation Program (NTPEP) laboratory for concrete admixture testing.

**Page 10-65, Article 1050-1, GENERAL, line 41**, replace the first sentence with the following:

All fencing material and accessories shall meet Section 106.

**\*\* NOTE \*\* Deleted revision to Page 10-73, Article 1056-1, DESCRIPTION, lines 7-8**

**\*\* NOTE \*\* Deleted revision to Page 10-73, Article 1056-2, HANDLING AND STORAGE, line 17**



**\*\* NOTE \*\*** Deleted revision to Page 10-73, Article 1056-4, GEOTEXTILES, line 33.

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**Addendum No. 3 January 5, 2016**

C203702 (I-3802B/I-3610/B-5365)

Standard Special Provisions

Cabarrus & Rowan Counties

**\*\* NOTE \*\* Deleted revision to Page 10-74, Table 1056-1, GEOTEXTILE REQUIREMENTS.**

**\*\*NOTE \*\* Deleted revision to Page 10-74, Article 1056-5, GEOCOMPOSITES, lines 7-8**

**The remaining portion of this page is intentionally left blank.**

In no instance shall a trainee be paid less than the local minimum wage. The Contractor shall adhere to the minimum hourly wage rate that will satisfy both the NC Department of Labor (NCDOL) and the Department.

**Achieving or Failing to Meet Training Goals**

The Contractor will be credited for each trainee employed by him on the contract work who is currently enrolled or becomes enrolled in an approved program and who receives training for at least 50 percent of the specific program requirement. Trainees will be allowed to be transferred between projects if required by the Contractor’s scheduled workload to meet training goals.

If a contractor fails to attain their training assignments for the calendar year, they may be taken off the NCDOT’s Bidders List.

**Measurement and Payment**

No compensation will be made for providing required training in accordance with these contract documents.

**ROCK AND BROKEN PAVEMENT FILLS**

(12-29-15)

235

DB2 R85

Revise the 2012 *Standard Specifications for Roads and Structures* as follows:

**Page 2-22, Article 235-2 MATERIALS**, add the following after line 19:

<b>Item</b>	<b>Section</b>
Geotextile for Rock and Broken Pavement Fills, Type 2	1056

Provide Type 2 geotextile for filtration geotextiles. Use rip rap and No. 57 stone from either a quarry or onsite material to fill voids in rock and broken pavement fills. Provide small and large size rip rap with stone sizes that meet Class A and B in accordance with Table 1042-1 and No. 57 stone with a gradation that meets Table 1005-1 or use similar size onsite material approved by the Engineer.

**Page 2-23, Subarticle 235-3(B) Embankment Formation**, lines 18-19, delete the third sentence in the seventh paragraph.

**Page 2-23, Subarticle 235-3(B) Embankment Formation**, lines 21-23, replace the eighth paragraph with the following:

Before placing embankment fill material or filtration geotextiles over rock and broken pavement, fill voids in the top of rock and broken pavement fill with rip rap and No. 57 stone. Place and compact larger rip rap first followed by smaller rip rap. Then, fill any remaining voids with No. 57 stone so geotextiles are not torn, ripped or otherwise damaged when installed and covered. Compact rip rap and No. 57 stone with tracked equipment or other approved methods. Install filtration geotextiles on top of rock, broken pavement, rip rap and No. 57 stone in accordance with Article 270-3 before placing remaining embankment fill material.

Remove any rocks, debris or pavement pieces from the roadbed larger than 2" within 12" of the subgrade or finished grade, whichever is lower.

## **GEOSYNTHETICS**

(12-29-15)

1056

DB10 R25

Revise the 2012 *Standard Specifications for Roads and Structures* as follows:

Replace Section 1056 with the following:

### **SECTION 1056 GEOSYNTHETICS**

#### **1056-1 DESCRIPTION**

Provide geosynthetics for subsurface drainage, separation, stabilization, reinforcement, erosion control, filtration and other applications in accordance with the contract. Use geotextiles, geocomposite drains and geocells that are on the NCDOT Approved Products List. Prefabricated geocomposite drains include sheet, strip and vertical drains (PVDs), i.e., "wick drains" consisting of a geotextile attached to and / or encapsulating a plastic drainage core. Geocells are comprised of ultrasonically welded polymer strips that when expanded form a 3D honeycomb grid that is typically filled with material to support vegetation.

If necessary or required, hold geotextiles and sheet drains in place with new wire staples, e.g., "sod staples" that meet Subarticle 1060-8(D) or new anchor pins. Use steel anchor pins with a diameter of at least 3/16" and a length of at least 18" and with a point at one end and a head at the other end that will retain a steel washer with an outside diameter of at least 1.5".

#### **1056-2 HANDLING AND STORING**

Load, transport, unload and store geosynthetics so geosynthetics are kept clean and free of damage. Label, ship and store geosynthetics in accordance with Section 7 of AASHTO M 288. Geosynthetics with defects, flaws, deterioration or damage shall be rejected. Do not unwrap geosynthetics until just before installation. Do not leave geosynthetics exposed for more than seven days before covering except for geosynthetics for temporary wall faces and erosion control.

**1056-3 CERTIFICATIONS**

Provide Type 1, Type 2 or Type 4 material certifications in accordance with Article 106-3 for geosynthetics. Define “minimum average roll value” (MARV) in accordance with ASTM D4439. Provide certifications with MARV for geosynthetic properties as required. Test geosynthetics using laboratories accredited by the Geosynthetic Accreditation Institute (GAI) to perform the required test methods. Sample geosynthetics in accordance with ASTM D4354.

**1056-4 GEOTEXTILES**

When required, sew geotextiles together in accordance with Article X1.1.4 of AASHTO M 288. Provide sewn seams with seam strengths meeting the required strengths for the geotextile type and class specified.

Provide geotextile types and classes in accordance with the contract. Geotextiles shall be identified by the product name printed directly on the geotextile. When geotextiles are not marked with a product name or marked with only a manufacturing plant identification code, geotextiles shall be identified by product labels attached to the geotextile wrapping. When identification is based on labels instead of markings, unwrap geotextiles just before use in the presence of the Engineer to confirm that the product labels on both ends of the outside of the geotextile outer wrapping match the labels affixed to both ends of the inside of the geotextile roll core. Partial geotextile rolls without the product name printed on the geotextile or product labels affixed to the geotextile roll core shall not be used.

Use woven or nonwoven geotextiles with properties that meet Table 1056-1. Define “machine direction” (MD) and “cross-machine direction” (CD) in accordance with ASTM D4439.

<b>TABLE 1056-1</b>						
<b>GEOTEXTILE REQUIREMENTS</b>						
<b>Property</b>	<b>Requirement</b>					<b>Test Method</b>
	<b>Type 1</b>	<b>Type 2</b>	<b>Type 3<sup>A</sup></b>	<b>Type 4</b>	<b>Type 5<sup>B</sup></b>	
<i>Typical Application</i>	<i>Shoulder Drains</i>	<i>Under Rip Rap</i>	<i>Silt Fence Fabric</i>	<i>Soil Stabilization</i>	<i>Temporary Walls</i>	
Elongation (MD & CD)	≥ 50%	≥ 50%	≤ 25%	< 50%	< 50%	ASTM D4632
Grab Strength (MD & CD)			100 lb <sup>C</sup>			ASTM D4632
Tear Strength (MD & CD)	Table 1 <sup>D</sup> , Class 3	Table 1 <sup>D</sup> , Class 1	—	Table 1 <sup>D</sup> , Class 3	—	ASTM D4533
Puncture Strength			—			ASTM D6241
Ultimate Tensile Strength (MD & CD)	—	—	—	—	2,400 lb/ft <sup>C</sup> (unless required otherwise in the contract)	ASTM D4595
Permittivity	Table 2 <sup>D</sup>	Table 6 <sup>D</sup>			0.20 sec <sup>-1,C</sup>	ASTM D4491
Apparent Opening Size	15% to 50% <i>in Situ</i> Soil	15% to 50% <i>in Situ</i> Soil	Table 7 <sup>D</sup>	Table 5 <sup>D</sup>	0.60 mm <sup>E</sup>	ASTM D4751
UV Stability (Retained Strength)	Passing 0.075 mm	Passing 0.075mm			70% <sup>C</sup> (after 500 hr of exposure)	ASTM D4355

- A. Minimum roll width of 36 inches required
- B. Minimum roll width of 13 feet required
- C. MARV per Article 1056-3
- D. AASHTO M 288
- E. Maximum average roll value

**1056-5 GEOCOMPOSITE DRAINS**

Provide geocomposite drain types in accordance with the contract and with properties that meet Table 1056-2.

<b>TABLE 1056-2</b>				
<b>GEOCOMPOSITE DRAIN REQUIREMENTS</b>				
<b>Property</b>	<b>Requirement</b>			<b>Test Method</b>
	<b>Sheet Drain</b>	<b>Strip Drain</b>	<b>Wick Drain</b>	
<b>Width</b>	≥ 12" (unless required otherwise in the contract)	12" ±1/4"	4" ±1/4"	N/A
<b>In-Plane Flow Rate<sup>A</sup></b> (with gradient of 1.0 and 24-hour seating period)	6 gpm/ft @ applied normal compressive stress of 10 psi	15 gpm/ft @ applied normal compressive stress of 7.26 psi	1.5 gpm <sup>B</sup> @ applied normal compressive stress of 40 psi	ASTM D4716

**A.** MARV per Article 1056-3

**B.** Per 4" drain width

For sheet and strip drains, use accessories (e.g., pipe outlets, connectors, fittings, etc.) recommended by the Drain Manufacturer. Provide sheet and strip drains with Type 1 geotextiles heat bonded or glued to HDPE, polypropylene or high impact polystyrene drainage cores that meet Table 1056-3.

<b>TABLE 1056-3</b>			
<b>DRAINAGE CORE REQUIREMENTS</b>			
<b>Property</b>	<b>Requirement (MARV)</b>		<b>Test Method</b>
	<b>Sheet Drain</b>	<b>Strip Drain</b>	
<b>Thickness</b>	1/4"	1"	ASTM D1777 or D5199
<b>Compressive Strength</b>	40 psi	30 psi	ASTM D6364

For wick drains with a geotextile wrapped around a corrugated drainage core and seamed to itself, use drainage cores with an ultimate tensile strength of at least 225 lb per 4-inch width in accordance with ASTM D4595 and geotextiles with properties that meet Table 1056-4.

<b>TABLE 1056-4</b>		
<b>WICK DRAIN GEOTEXTILE REQUIREMENTS</b>		
<b>Property</b>	<b>Requirement</b>	<b>Test Method</b>
Elongation	≥ 50%	ASTM D4632
Grab Strength	Table 1 <sup>A</sup> , Class 3	ASTM D4632
Tear Strength		ASTM D4533
Puncture Strength		ASTM D6241
Permittivity	0.7 sec <sup>-1,B</sup>	ASTM D4491
Apparent Opening Size (AOS)	Table 2 <sup>A</sup> ,	ASTM D4751
UV Stability (Retained Strength)	> 50% <i>in Situ</i> Soil Passing 0.075 mm	ASTM D4355

**A.** AASHTO M 288

**B.** MARV per Article 1056-3

For wick drains with a geotextile fused to both faces of a corrugated drainage core along the peaks of the corrugations, use wick drains with an ultimate tensile strength of at least 1,650 lb/ft in accordance with ASTM D4595 and geotextiles with a permittivity, AOS and UV stability that meet Table 1056-4.

**1056-6 GEOCELLS**

Geocells shall be identified by product labels attached to the geocell wrapping. Unwrap geocells just before use in the presence of the Engineer. Previously opened geocell products shall be rejected.

Manufacture geocells from virgin polyethylene resin with no more than 10% rework, also called “regrind”, materials. Use geocells made from textured and perforated HDPE strips with an open area of 10% to 20% and properties that meet Table 1056-5.



<b>TABLE 1056-5 GEOCELL REQUIREMENTS</b>		
<b>Property</b>	<b>Minimum Requirement</b>	<b>Test Method</b>
Cell Depth	4"	N/A
Sheet Thickness	50 mil -5%, +10%	ASTM D5199
Density	58.4 lb/cf	ASTM D1505
Carbon Black Content	1.5%	ASTM D1603 or D4218
ESCR <sup>A</sup>	5000 hr	ASTM D1693
Coefficient of Direct Sliding (with material that meets AASHTO M 145 for soil classification A-2)	0.85	ASTM D5321
Short-Term Seam (Peel) Strength (for 4" seam)	320 lb	USACE <sup>C</sup> Technical Report GL-86-19, Appendix A
Long-Term Seam (Hang) Strength <sup>B</sup> (for 4" seam)	160 lb	

**A.** Environmental Stress Crack Resistance

**B.** Minimum test period of 168 hr with a temperature change from 74°F to 130°F in 1-hour cycles

**C.** US Army Corps of Engineers

Provide geocell accessories (e.g., stakes, pins, clips, staples, rings, tendons, anchors, deadmen, etc.) recommended by the Geocell Manufacturer.